



Finite Element Analysis (FEA) Based Frequency Optimization of Vertical Storage Column

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Abstract

The ethanol containing vertical storage column is typically a leak proof tank which is used to store the liquid and fine powder. To exploit the storing capability, these columns are generally precise high normally above 27 meters with a circular cross section. The task arranged is that the agitator and the vertical pole are necessary to be on the equal podium. Due to this the pole is predisposed to vibrations from the Campaigner. This was needed to optimize the natural frequency of the column. Stiffeners /Support are fond of to the column plate to reinforce the column counter to pressure force, to minimize the vibrations of the plate and to become rigid plate against buckling. Stiffeners must be set apart at some suitable positioning so that plate stress is low than permissible stress. If column panel tallness or breadth is less than acceptable plate span then according to assumption column doesn't requires stiffeners/support. Though if column lifted or handles in one piece then, some stiffening may be required to keep column shape. Rise the stiffness by growing stiffeners/supports leads to load increase so no actual variance in rate of recurrence. By reducing mass of column and that can be possible only by selecting proper material. The objective of entire effort is to design of the vertical column in such a way that the natural frequency of the column can be optimized. As number of supports(horizontal) are increasing the natural frequency of column is also increasing, but at same time weight is also increased. So in order to optimize design the weight of column provided by support must be reduced. After optimize the structure of support and carrying out pre stressed modal analysis the targeted natural frequencies (>16 Hz) is obtained in the structure.



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Introduction

The vertical storage column is typically a leak proof tank which is used to store the liquid and fine powder. The column which is used in this case is used to store ethanol. To expand the capacity limit, these segments are generally exceptionally tall commonly over 27 meters with a circular cross segment. The test presented is that the agitator and the vertical section are needed to be on a similar stage. Because of this the Column is powerless to vibrations from the Agitator. The frequency created by the agitator is legitimately relative to the revolutions every moment (rpm). On the off chance that the frequency produced by the agitator matches with the natural frequency of the Vertical Column then such a resonance will make the section vibrate at max adequacy and may even bring to Failure. The issue can be settled by expanding the characteristic natural frequency of the section. Natural frequency is a component of the mass (m) and the solidness (k)/in the event that we enhance these boundaries utilizing basic alterations we can expand Natural frequency and afterward we can work the agitator at higher rpm minimizing the creation time C . Design target is to improve production by at least 20. This will need to optimize the natural frequency of the column. Stiffeners/Support are fond to the column plate to reinforce the column compared to pressure loads, to decrease vibrations of the plate and to stiffen plate compared to buckling. Stiffeners essential be spaced at some appropriate spacing so that plate stress is less than allowable stress. If column panel height or width is less than allowable plate span then theoretically column does not requires stiffeners/support. However if column lifted or handles in one piece then, some stiffening may be necessary to maintain column shape. In any case stiffener /support become integral part of column's structural system. Stiffener/Support may also function as column supports as well as plate reinforcement. Stiffeners¹⁻² are generally most part welded to the segment divider are different techniques to join.

Increase the stiffness by increasing stiffeners/supports leads to weight increase so no real difference in frequency. By reducing mass of column and that can be possible only by selecting proper material. The objective of entire work is to design of

the vertical column in such a way that the natural frequency of the column can be optimized.

Literature review

2010 ASME Codes Section-VIII Division-II.³ ASME is the most well up in standard creating associations in America. It delivers around 600 codes and norms covering many specialized zones, for example, clip, plumbing connections, lifts, pipelines, and force plant frameworks and parts. ASME's standards are created by advisory groups of topic specialists utilizing an open, agreement based cycle. Many ASME standards are referred to by government organizations as apparatuses to meet their administrative purposes. ASME principles are consequently considered, except if the standards have been fused into a lawfully restricting business contract or joined into guidelines upheld by an authority having purview, for example, an administrative, state, or neighborhood government organization. ASME's principles are utilized in excess of 100 nations and have been converted into various languages. The biggest ASME standard, both in size and in the quantity of volunteers engaged with its arrangement, is the ASME Boiler and Pressure Vessel Code (BPVC). The BPVC gives rules to the plan, creation, establishment, examination, care, and utilization of boiler, pressure vessels and atomic parts. The code likewise remembers norms for materials, welding and brazing techniques and capabilities, nondestructive assessment, and atomic in-administration review.

W. F. English *et al.*,⁴ developed "fatigue Design Criterion For Pressure Vessel Alloys". The researcher have brought up two crucial issues which they have not replied. To begin with, for what reason should the 1961 mean bend, which precisely spoke to 146 information focuses somewhere in the range of 10 and 108 cycles, be altogether change. Also, what are the specialized defenses for stretching out the mean bend to 10s by utilizing a generally modest number of burden controlled tests? Changing a set up mean bend, having fantastic information fit, a huge mathematical information base, and 15 years of fruitful experience requires solid avocation. The creators' model and ward variable were equivalent to Langer's. Once more, why the proposed changes to the current Langer bend, the essential explanation

has all the earmarks of being the way wherein the information is fit to the model.

Y. Bangash,⁵ has proposed "Safe Analysis For Cooling Pipes For Prestressed Concrete Reactor Vessels". A bit by bit investigation is given for the immediate calculation of two-dimensional warmth stream to and safe pitching of the cooling pipes. Two models of the cooling framework have been chosen and computations have been done for a current vessel.⁶ On one model this investigation is contrasted and the three-dimensional limited component examination for getting protection conductance for different cooling pipe pitches.

WanminHant *et al.*,⁷ has proposed "Direct Vibration Analysis of Laminated Rectangular Plates Using the Hierarchical Finite Element Method-I. Vibration examination of evenly overlaid, rectangular plates with cinched limit conditions is considered utilizing the progressive limited component strategy. Emblematic calculation is utilized to assess the integrals of the results of expected higher request polynomials that happen when processing the inactivity and solidness networks.

Roman W. Motriuk *et al.*,⁸ has proposed technique for "Quick, Wide-Field Measurements of Complex Transient Shell Vibrations". The planning and assessment of complex vibration fields is frequently exceptionally attractive in the weight vessel and channeling industry. It is likewise dull and costly utilizing traditional innovation, for example, strain gage and accelerometer clusters. This paper portrays field and research center estimations made with a compact beat laser framework that immediately catches uprooting information over territories up to 2 m², with submicron affectability. An extra objective is to expand the productivity of clamor reduction arrangements utilizing understanding got from wide-field vibration estimations.

M.H.Toorani,⁹ has introduced "Elements of The Geometrically Non-Linear Analysis of Anisotropic Laminated Cylindrical Shells". An overall methodology, in view of shearable shell hypothesis, to foresee the impact of mathematical non-linearities on the common frequencies of a versatile anisotropic overlaid round and hollow shell fusing enormous

removals and pivots is introduced in his work.

You-Hong Liu *et al.*,¹⁰ has watched "Breaking point Pressure and Design Criterion of Cylindrical Pressure Vessels with Nozzles". Breaking point pressures and comparing most extreme nearby film Stress Concentration Factors (SCF) are surveyed for two symmetrically meeting slender walled barrel shaped shells exposed to inward weight. The plastic breakdown pressures got by 3D FEM are in acceptable concurrence with test results introduced by past creators. The nearby layer SCF at the convergences of two barrel shaped shells exposed as far as possible weight load is determined by versatile meager shell hypothetical arrangements introduced by Xue and Hwang.

JaroslavMackerle,¹¹ has introduced "Limited Elements In The Analysis Of Pressure Vessels And Piping, An Addendum". It gives a bibliographical audit of limited component strategies (FEMs) applied for the investigation of weight vessel structures/ parts and channeling from the hypothetical just as functional perspectives. This list of sources is another addendum to the Finite components in the examination of weight vessels and channeling—a book index¹⁻³.

By completing above literature survey it is discovered that the vessel that is situated vertically, there is an possibility of twisting. At the point when the height of vessel is so enormous the above issue gets critical. So in such cases it is important to complete dynamic analysis of vessel in addition to structure investigation.

FAE Analysis

The little size nozzle, channel outlet pipe and other mounting and accessories are not consider with the work of pre-stressed modal analysis.(Because the heaviness of these devices are little contrasted and weight of the whole segment). The vertical storage segment is considered as thin pressure vessel in light of (diameter to thickness ratio is greater than 20). The 3D model is coincided with first order shell component (SHEEL 63). The every one of vertical help are fix into singular solid section. For static investigation the welding isn't need to recreate on the grounds that the weld material and parent material are thought to be same (and the pressure

is free from the material).therefore whole structure is consider as a ceaseless structure.

The structural analysis of the column is carried out in order to check whether the column structure is safe or not under the given boundary condition. Due to the presence of contact at the junction of reinforcement pad to the cylindrical shell of column the analysis become non linear static structural analysis.Due to large structure of column the weight of column is taken into account by applying gravity to the structure.The weight of the ethanol is simulated by scaling the density of structure.

Model of Vertical Storage Column

Following figure shows vertical solid pressure vessel Assembly design with two input nozzle which is at horizontal position & one output nozzle which is at lower portion of the vessel. Tube-sheet is mounted in the Shell. Initially wegenerate solid in Workbench model by using all calculated constraints and ASME Code Section-VIII, Div-II, and next meshing the solid model extra nodes are produced but ability of system not enough, so generate next model in surface. In surface modelling no. of nodes are cuts 40-60 % as match to solid model, which are beneath the ability to my system.

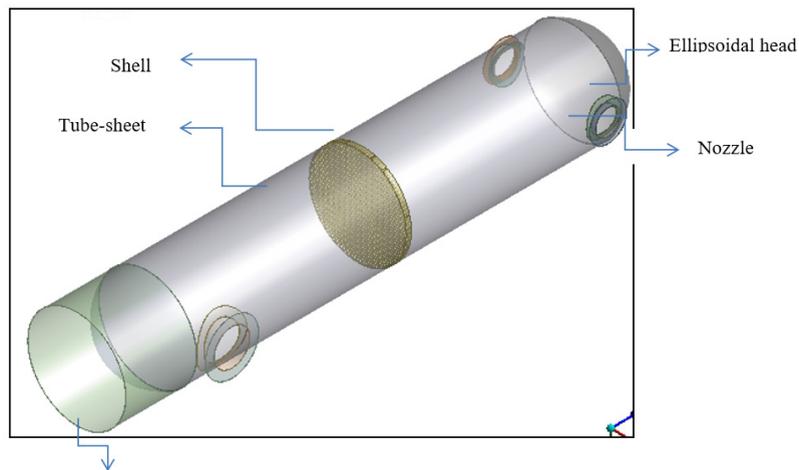


Fig. 1: Solid model of pressure vessel

Material used for the pressure vessel is SA516GR70 Steel are use. The properties of the material are described in subsequent table by using Reference of ASME Code Section-VIII, Div-I. Page no.50Basic methodology of finite element method is to prepare calculations at only restricted number of points and then interpolate the results for entire domain i.e. surface and volume. Steps Involved in Meshing Pressure Vessel in ANSYS are selection of element type for meshing and creating simplified parts & meshing the parts.

shapes are quadratic in both in-plane directions. The component has plasticity, stress stiffening, large deflection, and large strain abilities.

SHELL93 is especially appropriate suited to model shells. The component has six degrees of opportunity at every node: interpretations in the nodal x-direction, y-direction, and z- directions and revolutions about the nodal x-axes, y-axes, and z-axes. The elongated



Fig. 2: Mesh Modified model

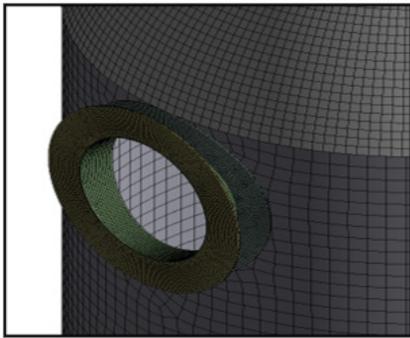


Fig. 3: Meshing at nozzle

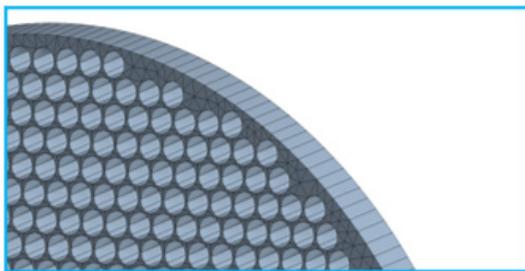


Fig. 4: Meshing on tube sheet

Static Structural Analysis

Applying Boundary Condition Are As Follows, For Pressure Vessel Analysis With Self Weight:

- Skirt support is fixed.
- Gravity acting downwards.

The gravity results display extreme deformation is 0.3419 mm and maximum stress is 10.82 MPa. At these vessel is safe for self-weight.

C: Static Structural_Gravity
Total Deformation
Type: Total Deformation
Unit: mm
Time: 1

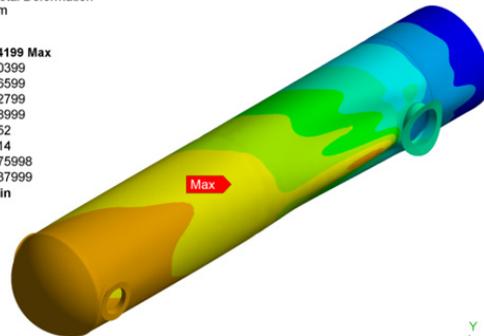
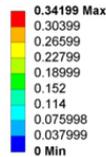


Fig. 6: maximum deformation=0.3419 mm

C: Static Structural_Gravity
Equivalent Stress
Type: Equivalent (von-Mises) Stress - Top/Bottom
Unit: MPa
Time: 1

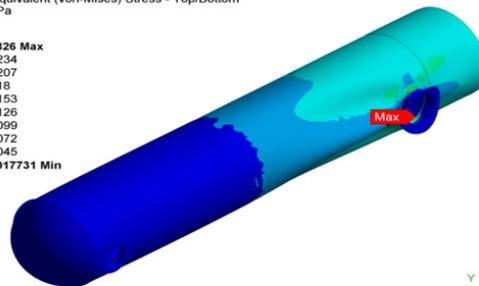
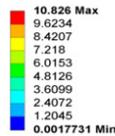


Fig. 7: Maximum stress = 10.826 MPa

Static Structural Analysis

Case-01

Applying Boundary Condition for Pressure Are As Follows,

- Apply Internal Pressure = 0.23MPa
- Wind load (Remote force) = 99792 N
- On Skirt Support applied Fix support constrain

C: Static Structural_Gravity
Static Structural
Time: 1. s
A Fixed Support
B Standard Earth Gravity: 9806.6 mm/s²



Fig. 5: Boundary condition

E: Static Structural_Wind Load+Pressure+Gravity
Static Structural 3
Time: 1. s

- A Fixed Support
- B Remote Force: 99792 N
- C Standard Earth Gravity: 9806.6 mm/s²
- D Pressure: -0.23 MPa
- E Pressure 2: 0.23 MPa
- F Pressure 3: -0.23 MPa
- G Pressure 4: -0.23 MPa
- H Pressure 5: -0.23 MPa
- I Pressure 6: -0.23 MPa
- J Pressure 7: -0.23 MPa



Fig. 8: Boundary conditions

Applying internal Pressure of 0.23 MPa on all bodies of pressure vessel & using remote force of 99792 N, which is acting on the circumference of the vessel.

From below chart & Table we did the meshing comparison at different node & element sizes

Table 2: Maximum Stress for Different number of Nodes

Sr. No.	No. of Nodes	Stress(mpa)	Deformation(mm)
1	60663	298.9	6.7177
2	91704	367.17	7.1943
3	120775	298.86	7.5601
4	149987	342.9	8.3247
5	179313	390.69	8.605
6	213726	324.68	8.7378
7	236546	333.88	8.9017

Case-02

This case is solved for the for the internal pressure to check maximum stress & deformation at 0.23 MPa pressure & gravity

- Internal pressure = 0.23 MPa
- Standard Earth gravity = 9.8 m/s²
- Applied fix constrain at skirt base

Boundary Condition

Use the same boundary condition of case-01 and with Standard Earth Gravity

D: Static Structural_Internal Pressure
 Static Structural 2
 Time: 1. s

- A** Fixed Support
- B** Pressure: -0.23 MPa
- C** Pressure 2: 0.23 MPa
- D** Pressure 3: -0.23 MPa
- E** Pressure 4: -0.23 MPa
- F** Pressure 5: -0.23 MPa
- G** Pressure 6: -0.23 MPa
- H** Pressure 7: -0.23 MPa
- I** Standard Earth Gravity: 9806.6 mm/s²

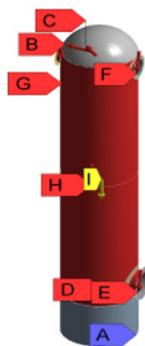


Fig.9: Boundary condition

D: Static Structural_Internal Pressure
 Total Deformation
 Type: Total Deformation
 Unit: mm
 Time: 1

7.2509 Max
 6.4453
 5.6396
 4.834
 4.0283
 3.2226
 2.417
 1.6113
 0.80566
 0 Min



Fig.10: Maximum deformation= 7.250 mm

D: Static Structural_Internal Pressure
 Equivalent Stress
 Type: Equivalent (von-Mises) Stress - Top/Bottom
 Unit: MPa
 Time: 1

368.23 Max
 137
 119.88
 102.77
 85.648
 68.531
 51.414
 34.297
 17.18
 0.062611 Min



Fig.11: Maximum stress=368.23 MPa

Case-03

For the next condition we have applied Internal Pressure 0.23, with wind load & standard gravity

Boundary Condition

- Internal pressure = 0.23 MPa
- Wind force = 99792 N
- Standard Earth gravity = 9.8 m/s²
- Applied fix constrain at skirt base

In above case the maximum deformation 9.5 mm in vertical pressure vessel although it is safe for self

weight because, maximum stress(von-mises) is less than total stress (i.e. membrane stress, $S +$ bending stress, $2S$) where $S =$ allowable stress, 137.89 MPa.

it is necessary to perform structure analysis first then same FEA model is forwarded for pre stressed modal analysis.

Therefore, vertical column is safe from structure point of view. In order to perform pre stress modal analysis,

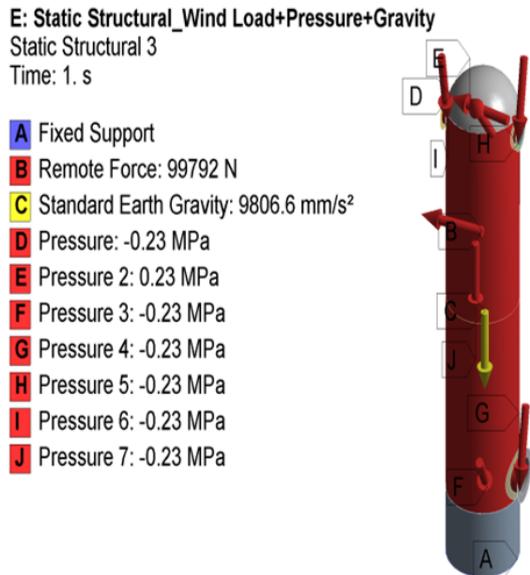


Fig.12: boundary Condition

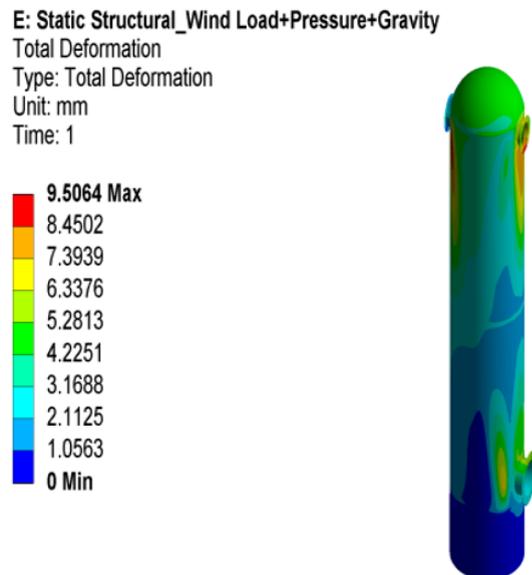


Fig.13: Maximum deformation= 9.5064 mm



Fig.14: Maximum stress=375.23 MPa

**Case 04
Solid Tube-sheet Optimization at pressure 0.23 MPa**

Applying boundary condition for optimization of single tube-sheet thickness

- Fixed support

- Rub in pressure= 0.23 MPa

Tube-sheet is investigated for pressure of 0.23MPa, reducing the tube-sheet thickness from 288 mm to achieved the peak thickness. The stress at 180 mm thickness of tube-sheet is 127.58 MPa which is nearby and minimum than permissible stress. So the final Optimum thickness for tube-sheet at pressure of 0.23 MPa is decided at 180mm

Table 4: Extreme Stress &Extreme Deformation results for Different Thickness of tube-sheet

Sr. No.	Thickness	Maximum Deformation (mm)	Maximum Stress (MPa)
1	288	1.2553	47.499
2	280	1.3307	50.397
3	260	1.6538	58.9
4	240	2.0922	69.705
5	220	2.7027	83.709
6	200	3.578	102.28
7	180	4.88	127.58

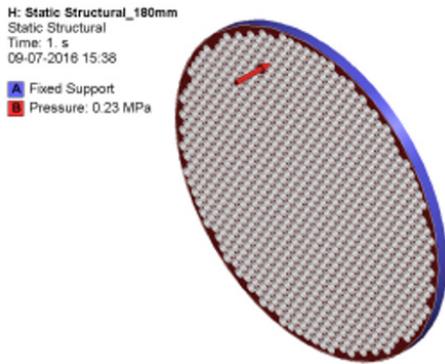


Fig.15: Boundary condition

Total mass of all tubes on the tube sheet = 2.5×923
 = 2307.5 kg

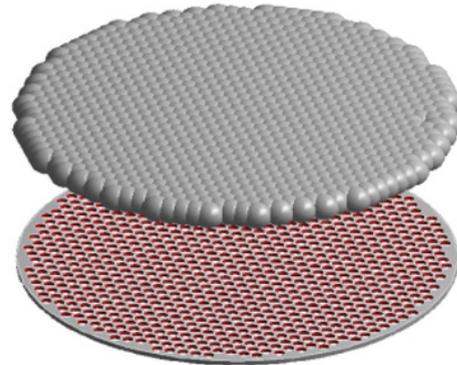


Fig.18: Point mass of 2.5 kg of every tube at its C.G

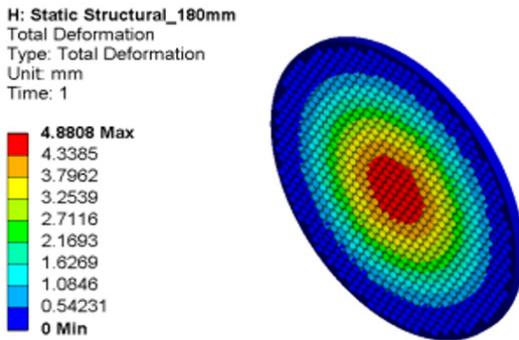


Fig.16: Maximum deformation= 4.88 mm

Table 5: Maximum Stress And Maximum Deformation results for Different Thickness of tube-sheet with mass of each tube at its C.G.

Sr. No.	Thickness	Maximum Deformation (mm)	Maximum Stress (MPa)
1	180	1.4419	37.686
2	160	1.9934	47.122
3	140	2.8833	60.782
4	120	4.4224	81.616
5	100	7.3412	102.82
6	90	9.8378	121.87
7	80	13.63	135.32

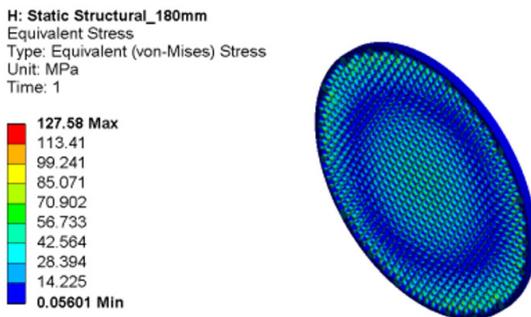


Fig.17: Maximum stress=127.58 MPa

Case 05

Tube-sheet Optimization with Point Load and Pressure of 0.05 MPa

Optimised 180 mm thickness of tube-sheet with rub on Gravity, point mass 2.5 kg of every tube on mid-point of tube length and pressure on tube-sheet 0.05 MPa, a number of analysis are as follows.

Total number of Tubes = 923
 Mass of each tube = 2.5 kg

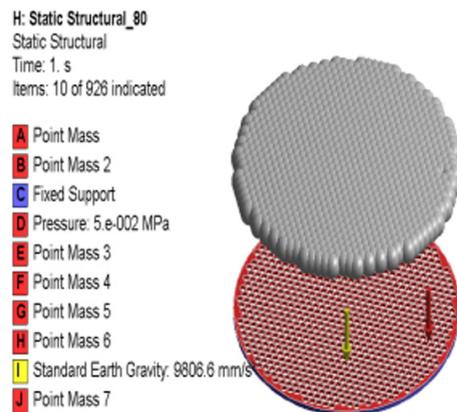


Fig.19: Boundary Condition

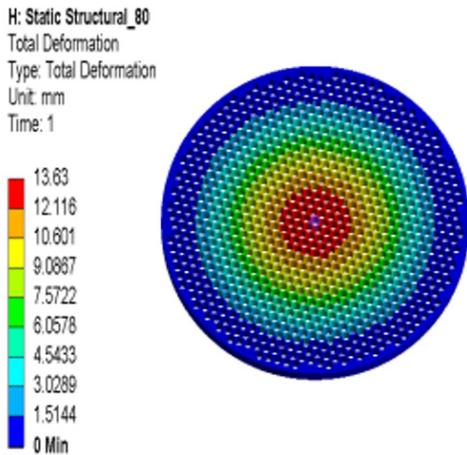


Fig. 20: Deformation =13.63 mm

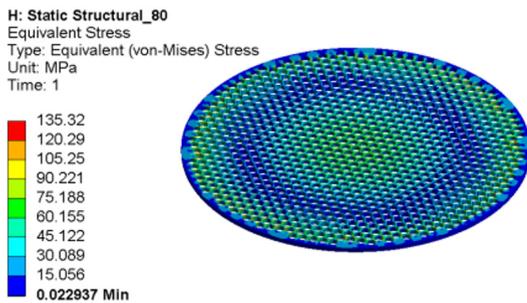


Fig.21: maximum stress= 135.32 MPa

Tube-sheet analysis with Point Load 2.5 kg of each tube and pressure of 0.05 MPa is done with decreasing the tube-sheet thickness to obtain the optimum thickness. The stress at 80 mm thickness of tube-sheet is 135.32 MPa which is nearest and less than allowable stress. So the final Optimum thickness for tube-sheet with Point Load of each tube and Pressure of 0.05MPa is decided at 80mm.

Pre-stressed Modal Analysis

A modal analysis defines the vibration characteristics (natural frequencies and equivalent mode shapes) of a structure or a machine element. The natural frequency of a structure is the frequency value at the structure naturally tends to vibrate if it is subjected to pulse. We increase the natural frequency by following ways:

- By increasing stiffness
- By reducing mass

Natural Frequency of Vertical Storage Vessel: Model without Support

Model without support and it is fix at bottom face is shown in fig.22.

The natural frequency of model without support is calculated as 5.4482 Hz that is shown in fig.22.

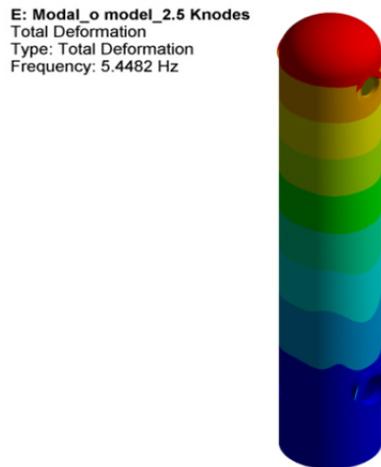


Fig: 22: Model without support and it is fix at bottom face

Model with Side Support: By using Plates of various Dimensions

1) With plate dimension 5000X5000X12mm
1.1) Five horizontal plates of height 8000 mm are attached to the one side of the model as shown in fig.23.

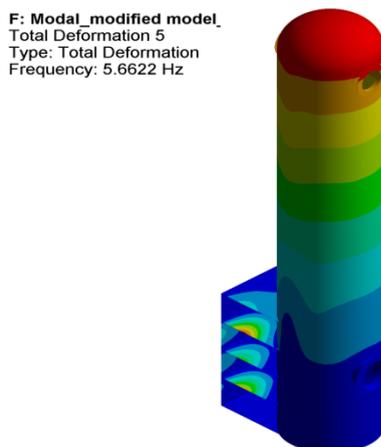


Fig: 23: Five horizontal plates of height 8000

The natural frequency of above model is increased to 5.6622 Hz as shown in fig. 29. Inclined plates are added in between two horizontal plates as shown in fig.30.

The natural frequency of above model is increased to 5.7609 Hz as shown in fig.24.

1.3)With 6 horizontal and 5 inclined plates and fix at bottom face as shown in fig.24

The natural frequency of above model is increased to 6.6272 Hz as shown in fig.25.

G: Modal_modified_02
Total Deformation - Mode 11 - 5.7609 Hz
Type: Total Deformation
Frequency: 5.7562 Hz

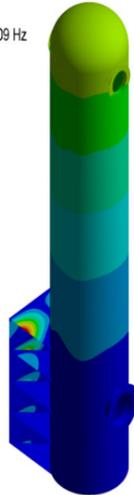


Fig: 24: Inclined plates are added in between two horizontal plates

Total Deformation - Mode 1 - 6.6272 Hz
Type: Total Deformation
Frequency: 6.1951 Hz

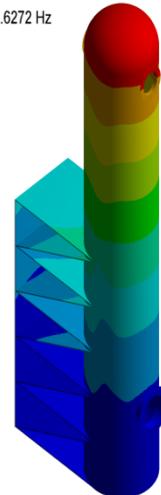


Fig: 25: With 6 horizontal and 5 inclined plates and fix at bottom face

2) With plate dimension 5000X5000X24mm

2.1) Two side support with 12 horizontal plates and 8 inclined plates as shown in fig.26.

The natural frequency of above model is increased to 7.1473 Hz as shown in fig. 26.

2.2) Three side support with 18 horizontal plates and 15 inclined plates as shown in fig.26

The natural frequency of above model is decreased to 6.8184 Hz as shown in fig. 27.

Total Deformation - Mode 1 - 7.1443 Hz
Type: Total Deformation
Frequency: 7.1443 Hz

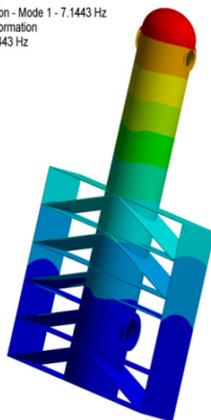


Fig: 26: Two side support with 12 horizontal plates and 8 inclined plates

Total Deformation - Mode 1 - 6.8184 Hz
Type: Total Deformation
Frequency: 6.8184 Hz



Fig: 27: Three side support with 18 horizontal plates and 15 inclined plates

3)With Plate Dimension 1000X3000X30mm

3.1) Three side support with 21 horizontal plates and 18 inclined plates and fix at bottom as shown in fig.28.

The natural frequency of above model is increased to 7.3623 Hz as shown in fig.28

Total Deformation 2
Type: Total Deformation
Frequency: 7.3623 Hz



Fig: 28: Three side support with 21 horizontal plates and 18 inclined plates

Boundary conditions are shown in fig.30 and maximum(von-mises) stress is shown in fig. 31.

By increasing height of primary and secondary support we got natural frequency of vertical storage

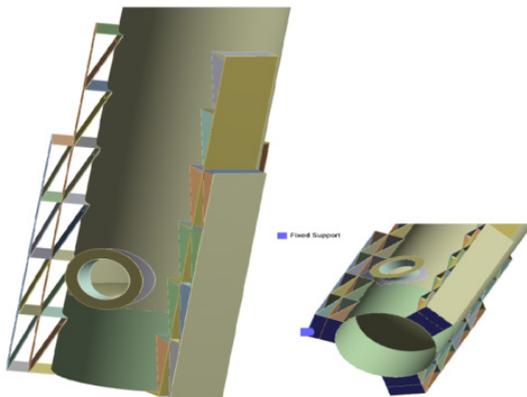


Fig: 30: Structural analysis of above model

3.2) Using primary and secondary support: As shown in fig.29.

The natural frequency of above model is increased to 9.5513 Hz as shown in fig. 29. Now, we increase the height of primary and secondary support with the same number of plates of same thickness as shown in fig.29.

Total Deformation - Mode 1 - 9.5513 Hz
Type: Total Deformation
Frequency: 9.5513 Hz

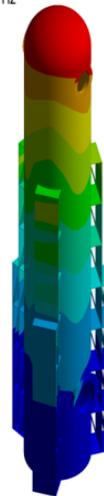


Fig: 29: Using primary and secondary support

column 18.03 Hz as shown in fig.6.45. Here, we got natural frequency of model is 18.03 Hz i.e. we got the targeted frequency which is greater than 16 Hz.

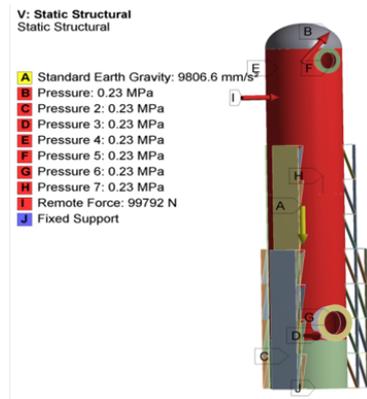


Fig: 31: Boundary conditions

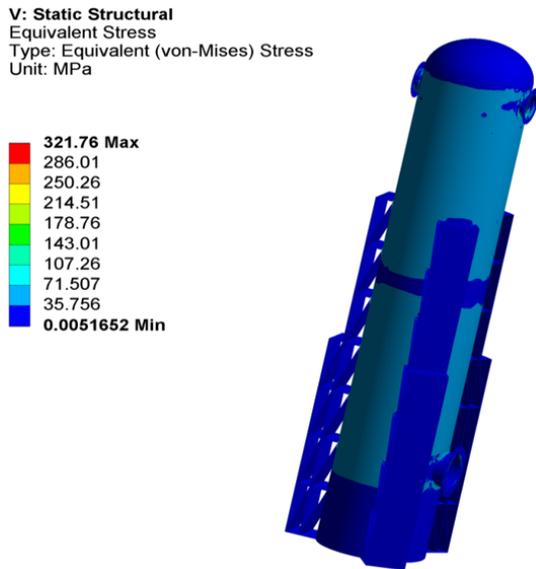


Fig: 32: Maximum stress=321.76MPa

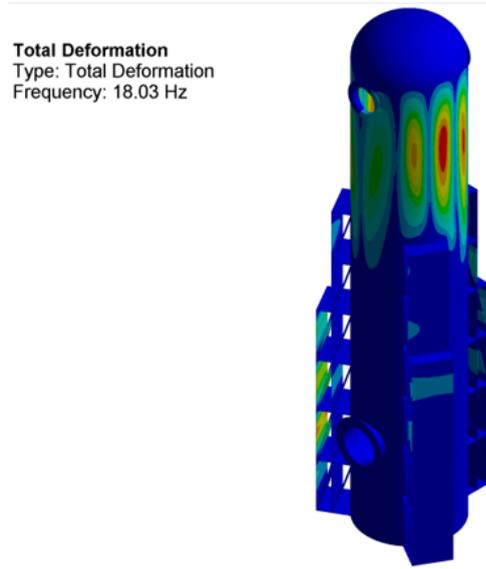


Fig: 33: Total deformation frequency

Results And Discussion

Result Obtained after Structure Analysis tube sheet

Solid Tube-sheet Optimization at Pressure 0.23 MPa

Tube-sheet is analysed used for pressure of 0.23 MPa, reducing the tube-sheet thickness from 288 mm to found the optimum thickness. The stress at 180 mm thickness of tube-sheet is 127.58 MPa which is close and less than permissible stress. So the concluding Optimum thickness for tube-sheet at pressure of 0.23 MPa is decided at 180mm.

Table 3: Maximum Stress And Maximum Deformation results for Different Thickness of tube-sheet

Sr. No.	Thickness	Maximum Deformation (mm)	Maximum Stress (MPa)
1	288	1.2553	47.499
2	280	1.3307	50.397
3	260	1.6538	58.9
4	240	2.0922	69.705
5	220	2.7027	83.709
6	200	3.578	102.28
7	180	4.88	127.58

Tube-sheet Optimization with Point Load and Pressure of 0.05 MPa

Tube-sheet analysis with Point Load 2.5 kg of each tube and pressure of 0.05MPa is complete with lessening the tube-sheet thickness to obtain the optimum thickness. The stress at 80 mm thickness of tube-sheet is 135.32 MPa which is nearest and less than allowable stress. So the final Optimum thickness for tube-sheet with Point Load of each tube and Pressure of 0.05MPa is decided at 80mm.

Table 4: Extreme Stress & Extreme Deformation results for Different Thickness of tube-sheet with mass of every tube at its C.G.

Sr. No.	Thickness	Maximum Deformation (mm)	Maximum Stress (MPa)
1	180	1.4419	37.686
2	160	1.9934	47.122
3	140	2.8833	60.782
4	120	4.4224	81.616
5	100	7.3412	102.82
6	90	9.8378	121.87
7	80	13.63	135.32

Result Obtained After Structure Analysis of Vertical Storage column

In case the maximum deformation 9.5 mm in vertical pressure vessel although it is safe for self weight because , maximum stress(von-mises) is less than total stress (i.e. membrane stress, S + bending stress, 2S) where S= allowable stress, 137.89 MPa. Maximum stress, 375.23 MPa < total stress, 413.67 MPa, Therefore, vertical column is safe from structure point of view.

In order to perform pre stress modal analysis, it is necessary to perform structure analysis first then same FEA model is forwarded for prestressed modal analysis.

Table 5: shows the maximum deformation and maximum stress with different boundry condition

Sr No	Boundary condition	Stress (MPa)	Dmx (mm)
1	Gravity+wind load	8.744	0.3463
2	Gravity+pressure	368.23	7.25
3	Gravity+pressure+ wind load	375.90	9.50

Pre Stressed Modal Analysis

With Plate Dimension 5000X5000X12mm

Natural frequency of vertical column 6.7906 Hz is received .so we have to increase the natural frequency by changing the plate thickness.

Table 6: The natural frequency of model with increasing support plates

Number of horizontal support	Number of inclined support	Height (mm)	Frequency (Hz)
5	-	8000	5.6662
5	4	8000	5.783
6	5	10000	6.6272
12	8	10000	6.6102
18	15	10000	6.7906

With Plate Dimension 5000X5000X24mm

Maximum natural frequency of vertical column 7.1443 Hz is recieved. so we have to increase the natural frequency by changing the plate thickness.

Table 7: shows the natural frequency of model with increasing support plates

Number of horizontal support	Number of inclined support	Height (mm)	Frequency (Hz)
5	4	15000	6.0721
6	5	18000	6.2772
12	8	18000	7.1443
18	15	18000	6.8184

With Plate Dimension 1000X3000X30mm

Maximum natural frequency of vertical column 7.3623 Hz is recieved. so we have to increase the natural frequency but we cannot increase the plate thickness more than 30mm so we can add secondary support to it as shown in fig.15.

Table 8: the natural frequency of model with increasing support plates

Number of horizontal support	Number of inclined support	Height	Frequency (Hz)
24	21	20000	7.3623

Using Primary and Secondary Support:

The frequency is 9.551 Hz so we have to increase the frequency by increasing height of primary and secondary support.

Table 9: the natural frequency of model with primary and secondary support plates

Number of horizontal support	Number of inclined support	Height	Frequency (Hz)
Primary -24	21	20000	9.5513
Secondary - 15	12	11428.14	

The frequency is 9.551 Hz so we have to increase the frequency by increasing height of primary and secondary support.

Table 10: shows the natural frequency of model with primary and secondary support plates

Number of horizontal support	Number of inclined support	Height (mm)	Frequency (Hz)
Primary -24	21	22000	18.03
Secondary - 15	12	12571.42	

Using Primary and Secondary Support

The output obtained from modal analysis are frequencies and mode shapes. In order to obtain percentage increase in frequency, comparison of fundamental frequency or first mode frequency of actual model of column with modified model of column to be carried out because agitator is only source of vibration. Our aim is to achieve fundamental frequency of model is greater than 16 Hz by optimize structure of column.

- As number of supports(horizontal) are increasing the natural frequency of column is also increasing, but at same time weight is also increased. So in order to optimize design the weight of column provided by support must be reduced.
- After optimize the structure of support and carrying out pre stressed modal analysis the targeted natural frequencies(>16 Hz) is obtained in the structure (as determine in FIG:5.18).

Conclusion

Structure Analysis

- The stress induced by applying given boundry condition in last case is less than permissible

or design stress value .

- All the cases exhibits stress generated is less than allowable or design stress.
- After carrying out structural analysis it is proved that the given model (i.e fig.6.42) is safe from strength and rigidity point of view.
- Selected model is forwarded for pre stressed modal analysis.

Pre-Stressed Modal Analysis

- As the number of support increases the natural frequency of column are increase but at the same time weight of the structure are also increase so the design are not optimize.
- In order to optimized the design it is necessary to reduce the weight of column as much as possible by maintaining same structural stiffness.
- By keeping in mind above consideration there are various model of column with different layout of support are developed. The last model as shown in fig.6.42 is achieving targeted natural frequency or more than and it is safe from structural point of view so we finalised that model.FEA analysis carried out in present work are very helpful in order to increase natural frequency of any vessles that is oriented vertically by adding some supports/stiffeners or making some structural modification.

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Conflict of Interest

The authors do not have any conflict of interest.

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